GROUNDWATER DEVELOPMENT

HATCH POINT, B.C.

FOR

CRC CANADIAN RETIREMENT CORPORATION

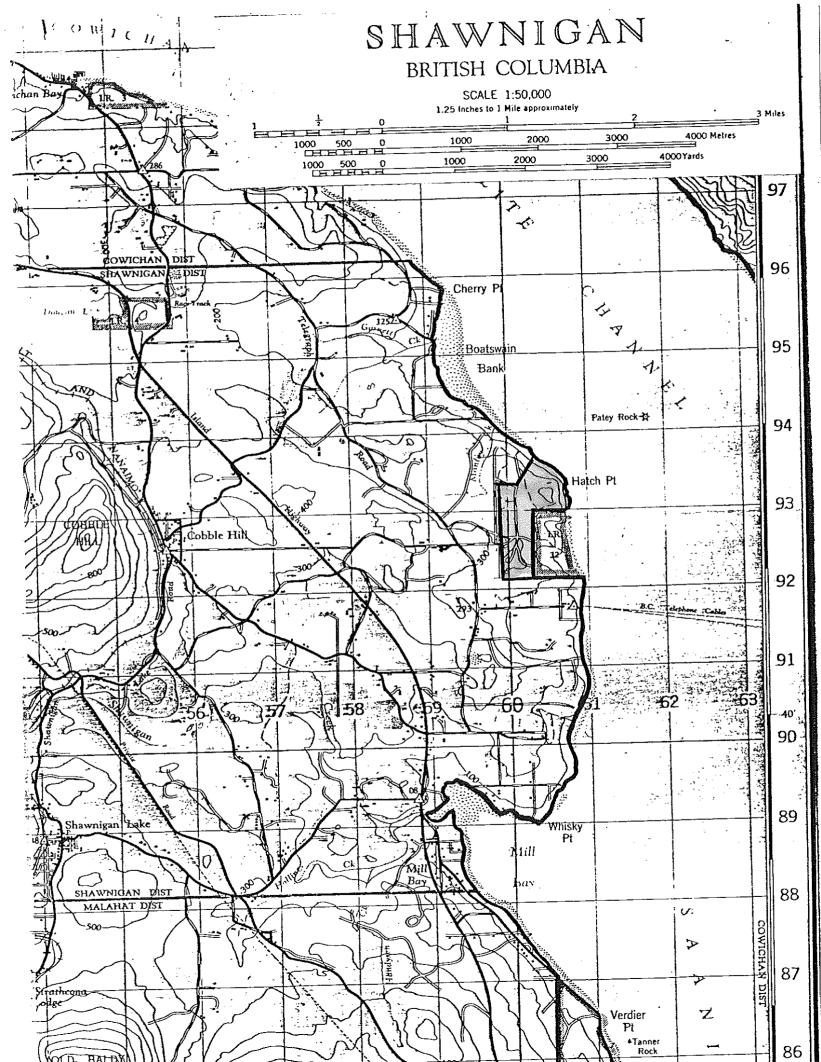
AND

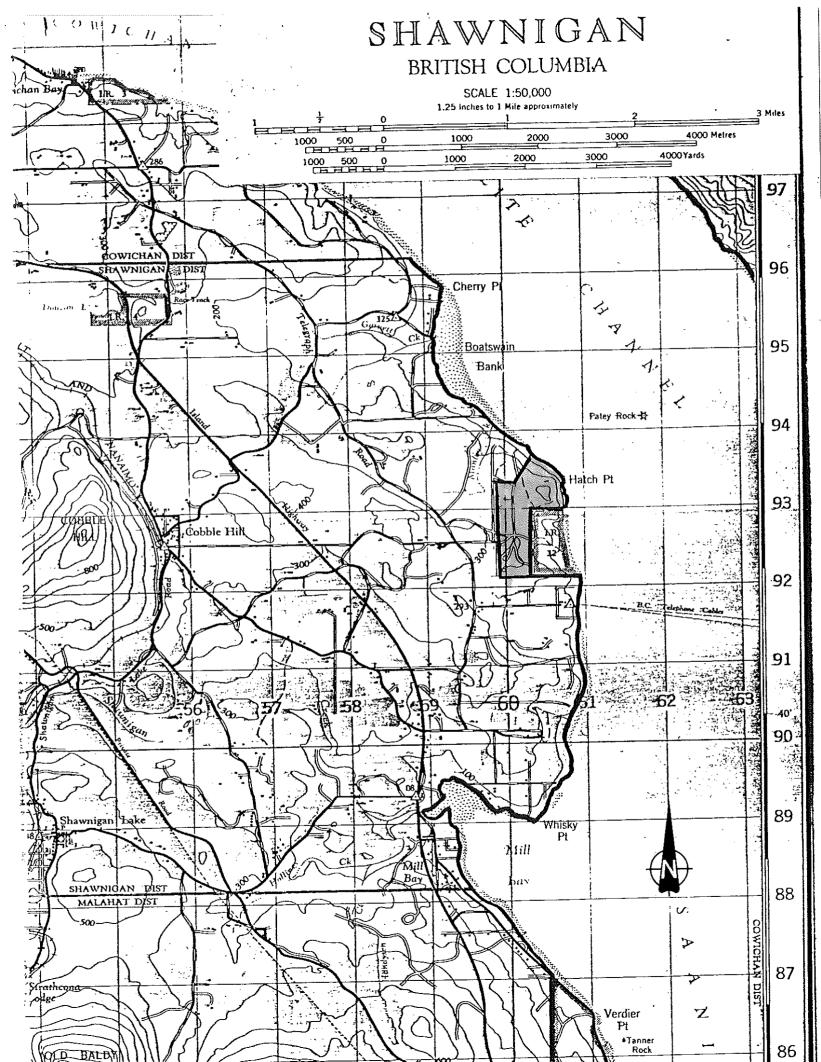
APLIN & MARTIN ENGINEERING LTD.

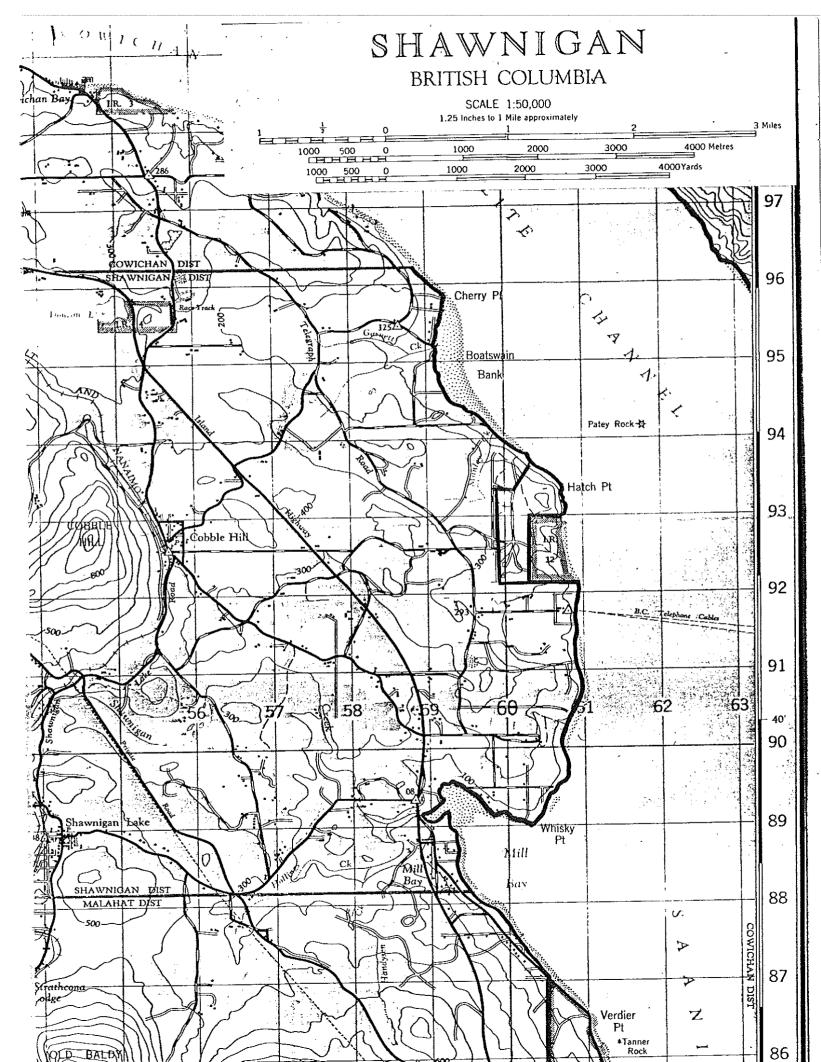
Test Well I Now Irrigation Well 1
2 abandoned No Aquifer
3 Now Domestic Well 1
4 Now Domestic Well 2

Ву

W.L. Brown, P. Eng.







1.0 INTRODUCTION

- 1.1 Location The attached copy of the northeastern part of the Shawnigan topographic map 92 B/12 East Half on a scale of 1:50,000 shows that the subject Hatch Point Area lies approximately 4,500 metres east of Cobble Hill and 4,000 metres north of Mill Bay. The area is bounded on the west by Ratcliffe Road and on the east by the coastline of Satallite Channel and I.R. No. 12. Please see the area coloured orange on the attached map.
- 1.2 Topography The western half of the area has a subdued rolling topography while the eastern part of the property falls steeply eastwards to the sea. The land surface rises to an elevation of 102 m in the southwestern part of the property.
- 1.3 Drilling and Testing The program started in September, 1985 and continued intermittently until June, 1987. A completion production pumping test is planned for the summer of 1987 and should be completed in September, 1987. This test will be conducted during the germination of the grass on the Golf Course when all wells will be pumped continuously at their rated capacities.

2.0 HYDROGEOLOGY (Please see the sections attached)

- Bedrock Sandstones and shales of the Comox Formation 2.1 These rock were encountered in Wells No. 1 and 2. types are observable close to ground surface in the northeastern part of the site and along the sea coast. Wells 3 and 4 encountered granitic bedrock of the This granitic bedrock was fractured Island Intrusions. did contain significant the fractures not quantities of groundwater. Similar fractured rocks contain appreciable amounts of groundwater across the inlet on Saanich Peninsula. For this reason 80 and 50 metres of the granitic bedrock was drilled and tested for groundwater content without success.
- 2.2 Unconsolidated Sediment Sands and gravels with interbeds of silts and clays overly a glacial till that lies on top of bedrock. The sands and gravels are water-bearing below depths of 30 to 45 metres. Two aquifers are present, an upper Water Table type aquifer and a lower Artesian Aquifer. The lower Artesian

Aquifer was only encountered in Well No. 1 where it lies below a clay bed which is approximately 12 metres thick.

2.3 Regional Considerations - The bedrock valley explored by the drilling program extends to the northwest from the Hatch Point site beneath the adjoining properties and the Granfield farm were an 82 metre deep well is completed in fine sand that reportedly is capable of producing approximately 900 m³/day (140 Igpm) of groundwater. The buried bedrock channel lies to the south of and trends sub-parallel to the present surface valley.

A large area of approximately 70 km² (27 sq. miles) is available to recharge this buried valley. The precipitation in the area has been estimated to be 940 mm (37-inches). Because of the soil types, the subdued topography, and the small amount of runoff it is estimated that 15% of the precipitation that falls on the area is available to recharge the aquifer. The groundwater recharge increment of the precipitation is therefore approximately 2.6 million cubic metres (688.5 million gallons) of water per year. On a 1,440 minutes per day, 365 days per year basis, 2.6 million cubic metres of water is equivalent to 7,000 m³/day (1,100 Igpm). Although this calculation does not indicate the amount of groundwater that is available to the Hatch Point wells it does indicate that these wells are within a large recharge area and so the wells should be reliable.

2.4 General Considerations - The pump tests should be reliable even though they were run in May, 1987 because there is approximately 30 m of "dry" sand above the aquifers and this particular year has had a very "dry" spring.

The ratings of Well 3 and 4 are based upon the minimum specific capacities (m^3 /day per metre of drawdown) calculated from the pump test results. Useable drawdown is rated at 75 and 70% of the total available drawdown (top of screen minus static water level).

WELL CONSTRUCTION 3.0

Well No. 1 was drilled and developed by an air-rotary rig. Wells 3 and 4 were drilled to the water table using an air-rotary rig, and drilled through the water-bearing zones by a cable-tool rig.

Reference to the logs of the wells and their construction details will show that:

- 1. Well No. 1 is screened in the sands and gravels that lie below the clay zone with 152 mm (6-inch) telescopic diameter screens that vary from 30 to 10 slot openings.
- 2. Wells No. 3 and 4 are screened in the upper aquifer with 203 mm (8-inch) telescopic diameter screens. 178 mm (7-inch) diameter blank casing forms a pump sump below the screen that is approximately 6 m (20 feet) long and a riser above the screen that is approximately 0.6 m (2 feet) long.

This constrution was used because of the low static water levels and the limited amount of Total Available Drawdown. A "shrouded" pump can be set in the sump below the screen and the water level drawndown to just above the screen. has been used successfully for construction dewatering wells which pump clear and clean water for many years.

PUMP TESTING 4.0

 $\ensuremath{\mathcal{IW}}\xspace \ensuremath{\mathcal{N}}\xspace \ensuremath{\mathcal{A}}$ Well No. 1. - The subject well was pump tested at a 4.1 constant rate of $654 \text{ m}^3/\text{day}$ (100 Igpm) for 1,600 minutes (almost 27 hours) on May 25/26, 1987. levels were measured and recorded during pumping and for 14.5 hours after the pump was turned off. Attached please find the pump test data including measurements made in Wells 3 and 4.3

Hatch Point Well No. 1 has been dedicated for Golf Course Irrigation because it is located very close to the sewage disposal field. It will be used for a 150 day period each year.

The subject well has the following components. see the attached log of the well.

Component	Depth			
	Metres	Feet		
8-inch casing	00.00 - 72.39	000 - 237.5		
6-inch telescopic diameter screen assembly	71.63 - 80.46	235 - 264		
Back fill	80.46 - 98.45	264 - 323		

The test pump was set with its intake at a depth of 68.691 m (225.4 feet) and the bottom of the motor at a depth of 69.771 m (229 feet). Such a setting allowed unrestricted flow from the screen assembly to the pump intake.

Reference to the pump test data will show that the static water level in the subject well was at a depth of 30.898 m (101.4 feet) on May 25, 1987. With the pump intake at a depth of 68.7 m (225.4 feet) the Total Available Drawdown was therefore 37.80 m (124 feet).

Reference to the semi-log graphs of the drawdown and recovery water level measurements will show that:

- 1. The water level was slightly less than 1.5 m (5 feet) above the pump intake when the pump test ended.
- The water was vortexing slightly as indicated by the "scallops" near the bottom of the last leg of the drawdown chart:
- 3. The drawdown and recovery graphs are almost identical indicating a properly run test.
- 4. The two graphs show the influence of negative boundaries as the "cone" around the well expands with time of pumping.
- 5. The effective transmissivity (field permeability) is shown on the last leg of the drawdown and top leg of the recovery charts to be $10.56 \text{ m}^2/\text{d}$ or 850 U.S.qpd/ft.
- 6. The Production Design chart is drawn from zero drawdown to the pump suction at a time of 150 days or 216,000 minutes. Using the effective

transmissivity of 10.56 m^2/d (850 U.S.gpd/ft.) a discharge rate can be calculated at 343 m^3/day (52.5 Igpm).

If the well is pumped at this rate the water level should reach pump suction after a pumping period of 150 days assuming:

- that there is no recharge to the aquifer during the 150 day period.
- that the "cone" of influence around the pumping well does not encounter any more negative boundaries.

These two assumptions should be reasonable because the irrigation demand occurs during the dry summer and fall growing season when negligible recharge should occur. The "cone" of influence was established during the pump test to be close to the pump suction where it will be after 150 days of pumping at the 325 m³/day (50 Igpm) rate. It therefore should have encountered all the various boundaries present.

- 4.2 Well No. 2 This well did not encounter a sufficient thickness of water-bearing sands to warrant screening, developing or testing.
- 4.3 Well No. 3 A constant rate pump test was conducted on May 5/6, 1987. A discharge rate of 233.303 m³/day (35.7 Igpm) was maintained for 1450 minutes or slightly over one day. Drawdown water levels were measured and recorded during pumping and recovery water levels were measured for one hour after pumping stopped.

Please see the attached log of this well, the pump test data and the semi-log plots of the drawdown and recovery water levels. Reference to these plots will show that:

- 1. the drawdown water levels essentially stabilized after the first 16 minutes of pumping at a depth of 46.437 m (152.4 feet).
- the drawdown water levels ranged from a depth of 46.377 m (152.16 feet) to 46.462 m (152.43 feet) from 16 minutes into the test to the end of the test at 1450 minutes. This 8.50 cm (3.35-inch) fluctuation was probably caused by slight pump surges.

- 3. the lowest depth to water reading at 46.462 m (152.43 feet) is equivalent to a drawdown of 46.462 43.246 or 3.216 m (10.55 feet). The minimum specific capacity of the well is therefore 233.303/3.216 equals 72.5 m³/day/m (35.7/10.55 equals 3.4 Igpm per foot of drawdown).
- 4. The transmissivity calculated from the recovery and drawdown charts approximates 108 m²/day (8,700 U.S.gpd/ft). This value indicates that the specific capacity of the well should be 76.7 m³/day/m (3.6 Igpm per foot of drawdown). This is close to the 72.5 m³/day/m (3.4 Igpm per foot of drawdown) minimum value calculated from the test.

The safe production capacity of Well No. 3 can be calculated as follows:

	Metres	Feet
Top of Screen Static Water Level	50.90 43.25	167.0 142.0
Total Available Drawdown Safety (approx. 70% TADD)	7.65 2.26	25.0 7.4
Total Useable Drawdown	5.39	17.6

The Safe Production Capacity is therefore

 $5.4 \text{ m} \times 73 \text{ m}^3/\text{d/m} = 395 \text{ m}^3/\text{d} \text{ or}$

17.6 feet x 3.4 igpm/ft = 60 igpm

The pump testing of Well No. 32 had no effect on Well No. 1. The reverse will also be true.

It will be noted that Well No. 3 has a 178 mm (7-inch) diameter sump below the 203 mm (8-inch) diameter telescopic screens. A "shrouded" submersible pump can therefore be set below the screens allowing the water level to be drawndown to the top of the screens during pumping. However, the pumping rate of 60 Igpm. should use only 70% of the Total Available Drawdown.

DW 2

4.4 Well No. 4 - A constant rate pump test was conducted on this well during May 12/13, 1987 at a discharge rate of 447 m³/day (68 Igpm). Drawdown Water level readings were taken and recorded during the 1450 minute (slightly over one day) period of pumping and recovery water level measurements were recorded for 60 minutes after pumping was stopped.

Please see attached the details of the pump test, the log of the well and the semi-log plots of the drawdown and recovery readings. Reference to these plots will show that:

- 1. the drawdown water levels stabilized after 125 minutes into the test with a gradual rise thereafter of 0.538 m or 1.77 feet. This rise is not caused by infiltration of the pumped water because this water was discharged through a hose into the ditch along Ratcliffe Road at a distance of 120 metres (400 feet). Since surges of sandy water were discharged at times during the test the rise in the water levels is attributed to well development by pumping.
- 2. the effective transmissivity of the aquifer is approximately $93 \text{ m}^2/\text{d}$ or 7800 U.s.qpd/ft.
- 3. the maximum drawdown during pumping was 40.031-32.470 or 7.561 m (24.8 feet). The minimum specific capacity of the well was therefore $59.12~\text{m}^3/\text{day}$ per metre of drawdown drawdown). (2.8 Igpm per foot of transmissivity value indicates that the specific capacity should approximate 69.80 m³/day per metre (3.25 Igpm per foot) of drawdown.

The Safe Production Capacity of Well No. 4 is calculated as follows:

	Metres	Feet
Top of Screen	39.62	130.0
Static Water Level	32.47	106.5
Total Available Drawdown	7.15	23.5
Safety (75% of TADD)	1.79	5.9
Salety (75% OI IADD)	1.79	5.9
mat - 3 - ** b-1 mark - 3 -	- o.c	
Total Useable Drawdown	5.36	17.6

The Safe Productive Capacity is therefore:

$$5.36 \times 59.12 \text{ m}^3/\text{d/m} = 317 \text{ m}^3/\text{d}$$

or $17.6 \times 2.8 \text{ Igpm/ft} = 50 \text{ Igpm}$

The pumping of Well No. 4 had no effect on Wells 1 or 3.

The production pump should be set in the sump $\underline{\text{below}}$ the screen to allow the water level to be drawndown to the top of the screen.

5.0 WATER QUALITY

The results of the chemical analyses run on samples of groundwater collected just before the end of the pumping tests are attached. These results show that the water meets the drinking water guidelines for the parameters tested.

The suitability of this water for other than drinking water purposes and its possible effects upon distribution and pumping components should be checked by specialists.

6.0 RECOMMENDATIONS AND CONCLUSIONS

6.1 Based upon presently available information the Safe Production Potentials of the Hatch Point Wells are:

Well No. 1 - 343 m³/day (52.5 Igpm) Irrigation 4/Well No. 2 - -- no aquifer Well No. 3 - 393 m³/day (60.0 Igpm) Pomestic 4/Well No. 4 - 317 m³/day (50.0 Igpm) Pomestic 4/Well No

- 6.2 Well No. 1 with its location close to the effluent disposal field has been dedicated to golf course irrigation even though the screened interval is separated from ground surface with 50 m of soil and 12 m of clay. This well will be pumped at its design rate for 150 days per year.
- 6.3 Water wells should not be:

Over-pumped Vibrated Raw-hided 6.4 A long term production well field test will be conducted for several months during the Summer and Fall of 1987 when water will be used for irrigation purposes during the germination and initial growing period of the golf course grass. Results of this test will establish the safe production capacity of the Hatch Point well field under adverse groundwater conditions of no recharge during a "dry" year.

WELL LOGS

HATCH POINT

WELL NO. 1 Irrigation Well No. 1

		J
DEPTH (Below Metres	w Ground Surface) Feet	DESCRIPTION
0 - 10.0	0 0 - 33	Gravel, clean
10.0 - 34.	0 33 - 111	Gravel, slightly silty
34.0 - 46.0	0 111 - 150	Sand, brown
46.0 - 50.	0 150 - 163	Sand, blue, silty
50.0 - 62.	5 163 - 205	Clay, silty
62.5 - 72.	0 205 - 236	Sand and gravel, silty, brown
72.0 - 75.	0 236 - 246	Gravel, clean, fine, water-bearing, static water level 30.898 m (101 feet)
75.0 - 81.0	0 246 - 265	Sand, medium to fine, clean, water-bearing
81.0 - 84.	0 265 - 275	Sand, fine, silty
84.0 - 86.	0 275 - 282	Sand, some gravel
86.0 - 93.	0 282 - 304	Sand, very fine, silty
93.0 - 94.	0 304 - 310	Clay, blue, sticky
94.0 - 98.	0 310 - 322	Till
98.0 - 98.	5 322 - 323	Bedrock, sandstone
Construction	n Details	metres feet
178 mm (7-ii 203 mm (8-ii	nch) diameter rise nch) telescopic	
Hole backfi	ater screens lled	72.54 - 80.47 238.0 - 264.0 80.47 - 98.45 264.0 - 323.0
0	kaimlaaa akaal 20	0 10 10 and 10 alob /bon bo

Screens - stainless steel, 30, 12, 12 and 10 slot (top to bottom)

HATCH POINT
WELL NO 2

Depth (Below Ground Surface)

Metres	Feet	Description		
0 - 0.9 0.9 - 6.1	0 - 3 3 - 20	Top soil, brown. Clay, very hard, tight, light brown		
6.1 - 14.0	20 - 46	Sand and gravel, cleaner with depth		
14.0 - 17.7	46 - 58	Sand, medium, clean, water-bearing at 50 feet		
17.7 - 26.5 26.5 - 28.3 28.3	58 - 57 87 - 93 93	Clay with gravel Silt, grey Total depth - conglomerate and shale		

Hole backfilled and abandoned

HATCH POINT

WELL NO. 3 Domestic Well No 1

рертн	(Below	Ground	Surface)
DREII	IDCION	OL Curra	Dun - ~ ~ /

DESCRIPTION

Metres		Feet	
0 -	46.6	0 - 153	Sand and gravel, silty to very silty, "dry"
46.6 -	57.0	153 - 187	Sand, brown water-bearing, static water level 43.246 m (142 feet)
57.0 -	66.5	187 - 218	Sand, silty with clay layers
66.5 -	75.9	218 - 249	Till
75.9 -	161.5	249 - 530	Granitic bedrock, fractured but "dry"

Construction Details - below ground surface

	metres	feet
203 mm (8-inch) diameter casing 178 mm (7-inch) diameter riser 203 mm (8-inch) telescopic	00.00 - 50.90 50.20 - 50.90	000.0 - 167.0 164.7 - 167.0
diamater screens 178 mm (7-inch) diameter sump Hole backfilled	50.90 - 57.00 57.00 - 63.09 63.09 - 161.54	167.0 - 187.0 187.0 - 207.0 207.0 - 530.0
Screens - stainless steel, 8,	10, 15 and 20	slot (top to

HATCH POINT

WELL NO. 4 Domestie Well 2

DEPTH	(Berow	Ground	Surrace)	
		_		

DESCRIPTION

Metres	reet	
0 - 36.6	0 - 120	Sand and gravel, very silty, "dry"
36.6 - 44.2	120 - 145	Sand, medium to fine grained, water-bearing, static water level 32.470 m (106.5 feet) below ground
44.2 - 52.4	145 - 172	Sand, fine grained, silty
52.4 - 66.4	172 - 218	Silt, clayey
66.4 - 73.2	218 - 240	Sand, fine grained, silt interbeds
73.2 - 76.8	240 - 252	Sand, fine grained, very silty
76.8 - 83.2	252 - 273	Clay, silty
83.2 - 89.3	273 - 293	Till
89.3 - 137.2	293 - 450	Granitic bedrock, "dry"

Construction Details - below ground surface

			metres		feet	
178 mm	(7-inch)	diameter casing diameter riser	00.00 - 39.01 -	39.62 39.62	000 - 130 128 - 130	
	diamater	telescopic screens diameter sump	39.62 - 44.20 -		130 - 145 145 - 165	
	•	loss steel 20				

Screens - stainless steel, 20, 12 and 12 slot. (top to bottom)

PUMPTEST

IW 1 Hatch Point - Well #1 Constant Rate Test 25/26 May, 1987

DD Μ.

30.433

31.102 31.513 31.832 32.137 32.370 32.695 33.459 33.682 33.881 34.081 34.272 34.437 34.981 35.079 35.215 35.339 35.445 35.587 36.091 36.116 36.222 36.317

 $Q = 120 \text{ U.S.gpm} = 654 \text{ m}^3/\text{day}$

Q - 120 (0.0.gpm	- 054 m / 0		 		
Time	Elapsed Time Min.	DTW*	DD** M.	Time	Elapsed Time Min.	DTW M.
25/05/87 15:30	0	30.898	0	23:50	500	61.331
	0.5	38.945	8.047	26/05/87		
:31	1	43.218	12.320	00:40	550	62.000
•	1.5	45.418	14.520	01:30	600	62.411
:32	2	46.600	15.702	02:20	650	62.730
	2.5	47.595	16.697	03:10	700	63.035
:33	3	48.218	17.320	04:00	750	63.268
	3.5	48.666	17.768	04:50	800	63.593
:34	4	49.000	18.102	05:40	850	64.357
	4.5	49.258	18.360	06:30	900	64.580
: 35	5	49.436	18.538	07:20	950	64.779
:36	5 6	49.758	18.860	08:10	1000	64.979
:37	7	50.039	19.141	09:00	1050	65.170
:38	7 8	50,238	19.340	09:50	1100	65.335
: 39	9	50.460	19.562	10:40	1150	65.879
15:40	10	50.670	19.772	11:30	1200	65.977
: 42	12	50.925	20.027	12:20	1250	66.113
: 44	14	51.164	20.266	13:10	1300	66.237
:46	16	51.376	20.478	14:00	1350	66.343
:48	18	51.570	20.672	14:50	1400	66.485
15:50	20	51.763	20.865	15:40	1450	66.989
: 55	25	52.115	21.217	16:30	1500	67.014
16:00	30	52.430	21.532	17:20	1550	67.120
:05	35	52.723	21.825	18:10	1600	67.215
16:10	40	52.957	22.059			
: 15	45	53.179	22.281		ı	
16:20	50	53.375	22.477	* DTW -	- Depth t	o Water
16:30	60	54.108	23.210	** DD -	- Drawdow	n n
16:40	70	54.430	23.532			
16:50	80	54.702	23.804			
17:00	90	55.202	24.304			
17:10	100	55.422	24.524			
	125	55.934	25.036			
18:00	150	56.368	25.470			
18:50	200	57.410	26.512			
19:40	250	58.380	27.482			
20:30	300	59.000	28.102			
21:20	350	59.880	28.982			
23:18	498	60:356	29.468 29.952			

Hatch Point - Well #1 Test Recovery 26/27 May, 1987

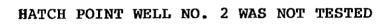
1	Recovery			,
	Time	DTW	DD	
Time	Min.	М.	М.	t/t'
18:10	0	67.215	36.317	
:11	1	58.754	27.856	1601
	1.5	55.353	24.455	1068
:12	2	53.289	22.391	801
	2.5	51.116	21.218	641
:13	3	51.284	20.386	534
	3.5	50.692	19.794	458
:14	4	50.277	19.379	401
	4.5	49.902	19.004	357
:15	5	49.622	18.724	321
:16	6	49.000	18.102	268
:17	7	48.678	17.780	230
: 18	. 8	48.425	17.527	201
:19	9	48.215	17.317	179
18:20	10	48.000	17.102	161
: 22	12	47.801	16.903	134
:24	14	47.550	16.652	115
:26	16	47.344	16.446	101
: 28	18	47.091	16.193	90
18:30	20	46.883	15.985	81
:35	25	46.313	15.415	65
18:40	30	46.018	15.120	54
:45	35	45.645	14.747	47
18:50	40	45.362	14.464	41
19:00	50	44.900	14.002	33
19:10	60	44.430	13.532	28
19:20	70	44.082	13.184	24
19:30	80	43.806	12.908	21
19:40	90	43.532	12.634	18.8
19:50	100	43.249	12.351	17
20:15	125	42.672	11.774	13.8
20:40	150	42.169	11.271	11.7
21:30	200	41.292	10.394	9
22:20	250	40.552	9.654	7.4
23:10	300	39.909	9.011	6.3
27/05/87	070	25 002	E 00E	2 04
08:40	870	35.983	5.085	2.84

Hatch Point - Well #1 Constant Rate Test - Observation Wells 25/26 May, 1987

		Elapsed	TW	TW	
		Time	No. 3	No. 4	- Measurements
	Time	Min.	DTW, m	DTW, m	top of casin
25/05/87	13:00	_	43.879	32.265	- Static Water
	15:00	_	43.876	20 250	
	15:30	0		32.259	
		1		32.259 32.259	
		3		32.259	
		2 3 4		32.259	
		5		32.259	
	15:40	10		32.258	
		15		32.258	
	15:50	20		32,257	
	16:00	30		32.257	
	16:10	40		32.257	
	16:20	50		32.258 32.258	
	16:30 16:50	60 80		32.258	
	17:10	100	<u> </u>	32.255	
	18:50	200	43.876	32.255	
	20:30	300		32.258	
	21:20	350		32.270	
	22:10	400	43.890		
0.0 / 0.0 / 0.0	23:00	450		32.270	
26/05/87	00:40	550	42 000	32.270	
	01:30 02:20	600 650	43.890	32.270	
	04:00	750		32.270	
	04:50	800	43.890	32.270	
	05:40	850		32.270	
	07:20	950		32.270	
	08:10	1000	43.890		
	09:00	1050		32.270	
	10:40	1150	43.904	32.284	
	11:30	1200	43.890	20 070	
	12:20	1250	42 000	32.270	
	13:10 14:00	1300 1350	43.890	32.280	
	14:50	1400	43.892	32.275	
	15:40	1450		32.276	
	16:30	1500	43.890	32.272	
ı	18:10	1600	43.888	32.269	
l l	. 1	. !	1	r I	

s from ngs

r Levels



Hatch Point Well No. 3

Constant Rate Test May 5/6, 1987 $Q = 42.8 \text{ U.S.gpm} = 233.303 \text{ m}^3/\text{d}$

<i>m:</i>	Elapsed Time Min.	DTW*	DD**
Time	LITII.	111 •	Fi •
11:00	0 0.5	43.246 45.026	0 1.780
:01	1 1.5	45.700 46.019	2.454
:02	2 2.5 3	46.165 46.258	2.919 3.012
:03	3 3.5	46.307 46.333	3.061 3.087
:04	4 4.5	46.357 46.371	3.111 3.125
: 05	5 6	46.391	3.145
:06	6	46.402	3.156
:07	7	46.408	3.162
:08 :09	8 9	46.410 46.420	3.164 3.174
: 10	10	46.427	3.181
:12	12	46.432	3.186
:14	14	46.441	3.195
:16	16	46.437	3.191
:18	18	46.435	3.189
: 20	20	46.435	3.189 3.187
: 25 : 30	25 30	46.433 46.430	3.184
: 35	35	46.431	3.185
:40	40	46.425	3.179
: 45	45	46.427	3.181
:50	50	46.426	3.180
12:00	60	46.426	3.180
: 10 : 20	70 80	46.425 46.422	3.179 3.176
: 30	90	46.401	3.155
: 40	100	46.403	3.157
13:05	125	46.406	3.160
13:30	150	46.377	3.131
14:20	200	46.387	3.141
15:10 16:00	250 300	46.379 46.387	3.133
16:50	350	46.388	3.141 3.142
17:40	400	46.388	3.142
18:30 19:20	450 500	46.387 46.387	3.141 3.141
		l i	I

Time	Elapsed Time Min.	DTW M.	DD M.
20:10 21:00 21:50 22:40 23:30 00:20 01:10 02:00 02:50 03:40 04:30 05:20 06:10 07:00 07:50 08:40 09:30 10:20	550 600 650 700 750 800 850 900 950 1000 1150 1200 1250 1350 1400	46.389 46.392 46.415 46.423 46.432 46.435 46.435 46.445 46.445 46.453 46.450 46.450 46.450 46.450 46.450	3.143 3.146 3.169 3.177 3.186 3.181 3.170 3.189 3.196 3.199 3.207 3.210 3.214 3.204 3.216 3.203 3.193
11:10	1450	46.412	3.166

^{*} DTW - Depth to Water ** DD - Drawdown

Hatch Point Well No. 3

3 Constant Rate Test May 5/6, 1987 Q = 42.8 U.S. gpm = 233.303 m³/d.

	Τ.	T		
	Recovery Time	DTW	I	DD
Time	Min.	М.	t/t'	М.
11:10	0	46.412		3.166
	0.5	44.791	2901	1.545
:11	1	44.042	1451	0.796
1	1.5	43.675	967.7	0.429
:12	2	43.546	726	0.300
	2.5	43.456	581	0.210
:13	3 3.5	43.427	484.3	0.181
	3.5	43.388	415.3	0.142
:14	4	43.381	363.5	0.135
	4.5	43.355	323.2	0.109
: 15	5	43.340	291	0.094
: 16	6	43.332	242.7	0.086
: 17	5 6 7 8	43.333	208.1	0.087
: 18	8	43.331	182.3	0.085
:19	9	43.313	162.1	0.067
: 20	10 .	43.306	146	0.060
: 22	12	43.302	121.83	0.056
: 24	14	43.298	104.67	0.052
: 26	16	43.300	91.63	0.054
: 28	18	43.298	81.56	0.052
:30	20	43.298	73.50	0.052
: 35	25	43.295	59	0.049
:40	30	43.285	49.33	0.039
: 45	35	43.282	42.43	0.036
:50	40	43.288	37.25	0.042
:55	45	43.286	33.22	0.040
12:00	50	43.282	30	0.036
12:10	60	43.279	25.17	0.033

 $p\omega$ 2 Hatch Point Well No. 4

Constant Rate Test May 12/13, 1987 Q = 82 U.S.gpm = 447 m^3d

Time	Elapsed Time Min.	DTW*	DD** M.		Time	Elapsed Time Min.	DTW M.	DD M.
10:00	0	32.470	0	•	17:30	450	39.822	7.352
	0.5	34.674	2.204		18:20	500	39.810	7.340
:01	1	36.110	3.640		19:10	550	39.782	7.312
	1.5	37.058	4.588		20:00	600	39.762	7.292
:02	2	37.552	5.082		20:50	650	39.735	7.265
	2.5	37.810	5.340		21:40	700	39.715	7.245
:03	3	38.000	5.530		22:30	750	39.702	7.232
	3.5	38.139	5.669		23:20	800	39.678	7.208
:04	4	38.188	5.718					
	4.5	38.240	5.770		00:10	850	39.650	7.180
:05	5	38.300	5.830		01:00	900	39.641	7.171
:06	6	38.342	5.872		01:50	950	39.622	7.152
:07	7	38.377	5.907		02:40	1000	39.613	7.143
:08	8	38.410	5.940		03:30	1050	39.599	7.129
:09	9	38.420	5.950		04:20	1100	39.566	7.096
:10	10	38.425	5.955		05:10	1150	39.588	7.118
:12	12	39.625	7.155		06:00	1200	39.547	7.077
: 14	14	39.754	7.284		06:50	1250	39.558	7.088
:16	16	39.815	7.345		07:40	1300	39.560	7.090
:18	18	39.754	7.284		08:30	1350	39.560	7.055
: 20	20	39.823	7.353		09:20	1400	39.518	7.048
: 25	25	39.870	7.400		10:10	1450	39.493	7.023
: 30	30	39.876	7.406		4 505			
: 35	35	39.908	7.438		* DTW *		o Water	
: 40	40	39.942	7.472		** DD	 Drawdow 	'n	
: 45	45	39.954	7.484 7.481					
:50 11:00	50 60	39.951 39.962	7.492					
11:10	70	39.984	7.492 7.514					
11:10	80	40.012	7.542					
11:30	90	40.008	7.538					
11:40	100	40.019	7.549					
12:05	125	40.030	7.560					
12:30	150	40.031	7.561					
13:20	200	40.015	7.545					
14:10	250	39.990	7.520					
15:00	300	39.920	7.450					
15:50								
16:40	350 400	39.871 39.840	7:401 7:370					

Hatch Point - Well #4 Test Recovery May 13, 1987

Time	Recovery Time Min.	DTW M.	DD M.	t/t'
10:10	0 0.5 1 1.5	39.493 36.221 34.490 33.635	7.023 3.751 2.020	2901 1451
:12	2 2.5	33.193 32.975	1.165 0.723 0.505	967.7 726 581
: 13	3 3.5	32.836 32.756	0.366 0.286	484.3 415.3
: 14	4.5	32.700 32.661	0.230 0.191	363.5 323.2
: 15 : 16 : 17	5 6 7 8	32.647 32.615 32.605	0.177 0.145 0.135	291 242.7 208.1
:18	8 9	32.600 32.585	0.130 0.115	182.3 162.1
10:20	10 12	32.575 32.566	0.105 0.096	146 121.8
: 24	14 16 18	32.567 32.575 32.570	0.097 0.105 0.100	104.6 91.63
: 28 : 30 : 35	20	32.562 32.540	0.092 0.070	81.56 73.50 59
: 40 : 45	30 35	32.538 32.550	0.068 0.080	49.33 42.43
:50 :55	40 45	32.554 32.540	0.084	37.25 33.22
11:00	50 60	32.550 32.540	0.080 0.070	30 25.17

Date

November 12, 1985

File No.

2355 A

Report On:

Water Analysis

Report To:

Brown & Erdmann 1409 Bewicke Avenue North Vancouver, B. C.

V7M 3C7

We have analysed the water sample submitted by you on October 29, 1985 and report as follows:-

SAMPLE INFORMATION

The sample was submitted in a proper laboratory container labelled:-

Hatch Point #1 Oct. 25/85 Temp 9.5°C

METHODOLOGY

The analyses were carried out using procedures described in "Standard Methods for the Examination of Water and Wastewater" published by the American Public Health Association, 1980.

RESULTS

See table following page.

REMARKS

The water as represented by the sample submitted can be characterized as moderate with respect to dissolved mineralization and hardness.

The water sample met British Columbia drinking water guidelines for all parameters analysed.

ASL ANALYTICAL SERVICE LABORATORIES LTD.

A. W. Maynard, M.Sc.

Senior Partner

AWM/mm



analytical service laboratories

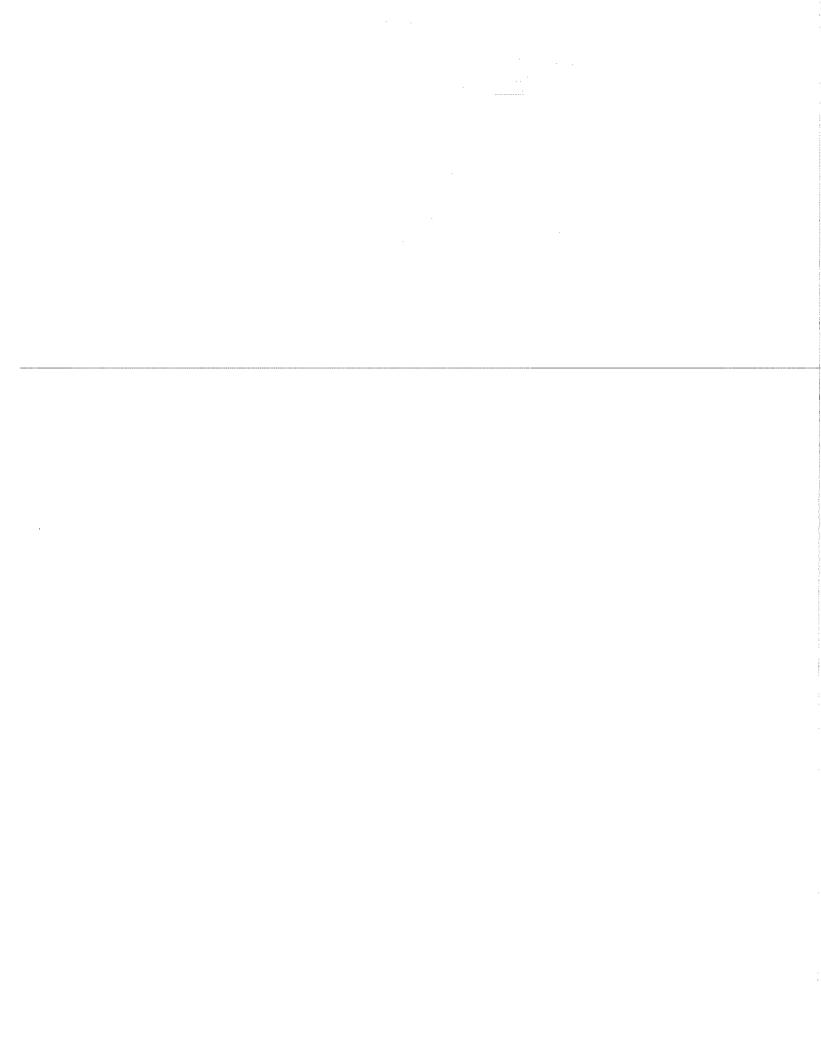
CONSULTING CHEMISTS & ANALYSTS
1650 Pandora Street
Vancouver, B.C. • V5L 1L6

RESULTS OF ANALYSIS Traigation Well Mo 1 Page 2 of 2

PARAMETER		SAM	PLE IDENTIFICA	ATION	Drinking
					Water Guidelines•1
Physical Parameters	5				6.5 - 8.5
pH Specific conductant	ce	7.62 153.			0.5 - 0.5
Color Turbidity	(umhos/cm) (CU) (JTU)	L5.0 L1.0			15. 5.
Suspended Solids Dissolved Solids Total Hardness	(mg/L) (mg/L) (mg/L)	5.0 101. 56.6			500.
Dissolved Anions (r	ng/L)		`		
Bicarbonate Carbonate Chloride Sulfate Fluoride	HCO ₃ CO ₃ CI SO ₄	73.2 - 1.96 ∟1.0 0.076			250. 500.
Nitrate + Nitrite Phosphate	N P	L0.003 0.07			10.0
Dissolved Metals (r	ng/L)		44.4 1.4.1		
Calcium Magnesium Sodium Potassium	Ca Mg Na K	14.2 5.14 5.89 0.82			
Cadmium Copper Lead Zinc	Cd Cu Pb Zn	L0.002 L0.005 L0.05 0.017			0.005 1.0 0.05 5.0
Iron Manganese	Fe Mn	L0.03 L0.005		•	
Total Metals (mg/l	_)				V.1
Iron Manganese	Fe Mn	L0.03 L0.005			0.3 0.05
	,				

L= Less than mg/L= milligrams per liter

^{&#}x27;1 "Maximum acceptable concentration" as published by Health & Welfare Canada, 1978



Domestic Well No I File No. 4134
Page 2 of 2 RESULTS OF ANALYSIS

Physical Para	imeters	Drinking *1 Water Guidelines	1
pH Conductivity (µ Colour Turbidity	umhos/cm) (CU) (NTU)	6.5-8.5 - 15. 5.	7.50 220. <5. <1.0
Total Hardness Dissolved Solid Suspended Solid	(mg/L) is (mg/L) is (mg/L)	(*2) 500. -	102. 178. <1.0
Dissolved Anior	ns (mg/L)		
Bicarbonate Carbonate Chloride Sulfate	HCO ₃ CO ₃ C1 SO ₄	250. 500.	7.50 6.2
Fluoride Nitrate + Nitri Silicate	ite N SiOz	1.5 10.0	0.083 1.10 3.9
Dissolved Metal	is (mg/L)		
Calcium Magnesium Sodium Potassium	Ca Mg Na K	~ (*3) ~	27.2 8.26 11.5 0.68
Iron Manganese	Fe Mn	_	<0.03 0.050
Arsenic Barium Cadmium Chromium	As Ba Cd Cr	0.05 1.0 0.005 0.05	0.0012 <0.005 <0.0002 <0.005
Copper Lead Zinc	Cu Pb Zn	1.0 0.05 5.0	<0.005 <0.001 0.014
Total Metals (n	ng/L)		
Iron Manganese	Fe Mn	0.30 0.05	<0.03 0.050



< = Less than
mg/L = milligrams per liter
*1 "Maximum acceptable concentration" as published by Health & Welfare Canada,
1985</pre>

Date:

June 9, 1987

File No.

4173A

ASL

Report On:

Water Analysis

Report To:

Brown Erdman & Turner Ltd.

1409 Bewicke Avenue North Vancouver, B. C.

V7M 2W0

We have analysed the water sample submitted by you on May 20, 1987 and report as follows:-

SAMPLE INFORMATION

The sample was submitted in proper laboratory container labelled:-

Hatch Point May 13/87 Test Well No. 4

METHODOLOGY

The analyses were carried out using procedures described in "Standard Methods for the Examination of Water and Wastewater" published by the American Public Health Association, 1985.

RESULTS

See attached table.

REMARKS

The water as represented by the sample submitted can be characterized as moderate with respect to dissolved mineralization.

The water sample met Canadian and British Columbia drinking water guidelines for all parameters analysed. Nitrate nitrogen was higher than normal and should be periodically checked.

ASL ANALYTICAL SERVICE LABORATORIES LTD.

A. W. Maynard, M.Sc. Senior Partner

AWM/mm



analytical service laboratories

CONSULTING CHEMISTS & ANALYSTS
1650 Pandora Street
Vancouver, B.C. • V5L 1L6
(604) 253-4188

RESULTS OF ANALYSIS Domestic Well No 2 File No. 4173A Page 2 of 2

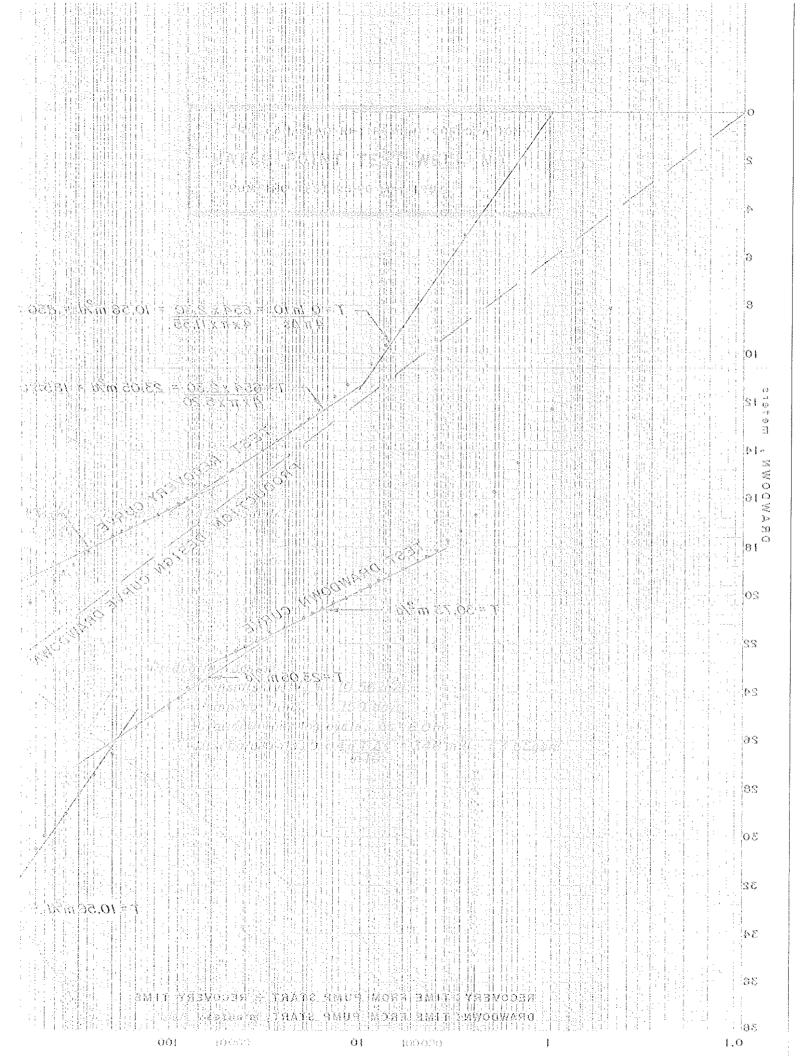
Physical Parameters	Drinking *1 Water Guidelines	Well #4
pH	6.5-8.5	6.95
Conductivity (µmhos/cm)	-	176.
Colour (CU)	15.	<5.
Turbidity (NTU)	5.	<1.0
Total Hardness (mg/L)	(*2)	90.8
Dissolved Solids (mg/L)	500.	170.
Suspended Solids (mg/L)	-	<1.0
Dissolved Anions (mg/L)	•	
Bicarbonate HCO ₃ Carbonate CO ₃ Chloride Cl Sulfate SO ₄	- 250. 500.	90.1 - 8.00 6.9
Fluoride F	1.5	0.13
Nitrate + Nitrite N	10.0	1.79
Silicate SiO ₌	-	22.1
Dissolved Metals (mg/L)		
Calcium Ca Magnesium Mg Sodium Na Potassium K	(E*)	23.7 7.59 5.33 1.05
Iron Fe		<0.03
Manganese Mn	-	0.011
Arsenic As	0.05	0.0010
Barium Ba	1.0	0.004
Cadmium Cd	0.005	<0.001
Chromium Cr	0.05	<0.010
Copper Cu	1.0	<0.005
Lead Pb	0.05	<0.001
Zinc Zn	5.0	0.013
Total Metals (mg/L)		
Iron Fe	0.30	<0.03
Manganese Mn	0.05	0.011

< = Less than
mg/L = milligrams per liter
*I "Maximum acceptable concentration" as published by Health & Welfare Canada,</pre>

^{*2 &}quot;Maximum level not established - water supplies with a hardness exceeding 200 mg/L are considered poor but will be tolerated. Not a health consideration

^{*3} Maximum level not established - of concern to consumers with sodium restricted diet. Levels exceeding 20 mg/L may be of concern in this circumstance

SEMI-LOG PLOTS



NO. 340-1410 DIETZGEN GRAPH PAPER SEMI-LOGARITHMIO 4 OYOLES X 10 DIVISIONS PER INCH

DIMTROMY DUNBURATION