LIVINGSTON, P. Eng.

PACIFIC HYDROLOGY CONSULTANTS LTD.

CONSULTING GROUNDWATER GEOLOGISTS

1401 WEST BROADWAY VANCOUVER, B.C. V6H 1H6 TELEPHONE: 738-9232

September 10, 1980

Rutland Waterworks District, 160 Highway 33 West, Kelowna, B.C. V1X 1X7

Attention: Mr. G.B. Sloane, Secretary-Treasurer

Dear Sirs,

Re: Construction and Testing of Well No. 11

The purpose of this letter-report is to put on record the results of the construction and testing of the District's new Well No. 11 located in the Works Yard between existing Wells No. 5 and No. 8. A dry testhole was drilled east of the Village before constructing the new well. The work was carried out by A.C. Drillers using a cable tool drill.

TEST DRILLING, WELL CONSTRUCTION AND TESTING

The log of the dry testhole is as follows:

0 1 ft. soil 1 37 ft. coarse dry bouldery gravel light grey clay (?) becoming darker 37 80 ft. 80 81 ft. sand with pebbles, making water 99 ft. 81 compact grey clay 99 _ 106 ft. grey sandy till (?) 106 108 ft. choclate brown clay shale -108 -109 ft. fresh granite rock

Following the drilling of the unsuccessful testhole and in light of decreased capacity of Well No. 5, the decision was made to construct a well in the pipe yard between existing Wells No. 5 and No. 8. Well No. 11 was drilled between the two wells, slightly closer to No. 5. Wells No. 5 and No. 8 are approximately 325 ft. apart.

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coarse gravel, cobbles, till (?) 0 17 ft. 17 23 ft. till, tan till (?), very silty, sandy, tan 23 43 ft. -43 62 ft. gravel, bouldery gravel, more sandy, looser with boulders 62 78 ft. _ at 78 ft. sand, gravelly, cobbly with water below 80 ft. 78 96 ft. ---sand, gravelly becoming grey at bottom 141 ft. 96 141 -143 ft. sand with silty interbeds 150 ft. 143 sand, gravelly gravel, very sandy, colour changes from 166 ft. 150 tan to dark brown silt, compact with thin sand interbeds 171 ft. 166 -

The static level at the time of testing was approximately 70 ft. below ground.

The log of Well No. 11 is as follows:

Drilling was started with 12 inch diameter casing. The well is completed as a 10 inch diameter well with 10" nominal diameter Johnson's stainless steel well screen as follows:

at top (142' 11")/4 ³	type K packer
2 ft.	0.030" slot screen
10 ft.	0.040" slot screen
2 ft.	0.030" slot screen
8 ft.	0.020" slot screen
at bottom (166 ft.)	bail bottom

The slot sizes were selected on the basis of sieve analyses of samples collected every few feet in the water-bearing sands and gravels. Copies of the analyses are appended. The 10" drive shoe is located at 143' 11" below ground.

Following well construction development by bailing was carried out for a short time. The water cleaned up quite quickly. Because of the difficulty with development of the two existing wells it was felt that vigorous development methods should be avoided.

Additional development by backwashing was carried out by Aqua-Flo following a short pump test. One day of backwashing improved the performance of Well No. 11 by 15-20%. Additional development is likely to result in further increases. The performance of Well No. 5 was improved considerably by backwashing for several days at the time of construction but it has gradually declined with use. Considerable

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time spent on redevelopment was unsuccessful in restoring original well performance. Well No. 8, however, performed considerably better at the time of construction and very little development was carried out.

Pump Testing

The pump testing and backwashing were carried out by Aqua-Flo Testing and Equipment. The purpose of the testing was to establish well performance; aquifer characteristics were known from testing of the existing wells. Data collected during the two short pump tests are included in the Appendix to this letter.

A short pump test of the new well to check the well performance was carried out on July 16 starting at 15:50. The two existing wells had been shut off at 08:45 in order that well interference could be monitored. The pumping level became stable quite quickly after the start of pumping so the test was terminated at 90 minutes and observations of the recovering water level were taken. The well performance of 5.37 USgpm/ft. of drawdown calculated at 90 minutes of pumping at 305 USgpm, was less than half the specific capacity of the poorest of the two existing wells, Well No. 5 , calculated at a pumping rate twice as high. Further, most of the drawdown occurs as well loss in the first few minutes after the start of pumping. For this reason, testing was terminated and backwashing carried out in an attempt to improve well efficiency.

Following about one day of backwashing the performance of Well No. 11 was 6.48 USgpm/ft. of drawdown calculated at 50 minutes of pumping at 314 USgpm. This shows that the backwashing was successful in improving well performance by about 15 to 20%. If further work to improve well performance is carried out, it should consist of methods other than backwashing. This is discussed later in this report.

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During the second test the pumps in Wells No. 5 and No. 8 were turned on one at a time to observe well interference. Sixty minutes after the start of pumping No. 11 at a rate of 314 USgpm, the drawdowns were as follows:

Well No.	ON or	Pump Rate	Water level	Drawdown
	OFF	(USgpm)	(ft.)	(ft.)
11	on	314	121.13	48.46
5	off		72.57	2.28
8	off		75.47	1.19

At 60 minutes after the start of pumping, Well No. 8 was turned on at 510 USgpm. After pumping Wells No. 11 and No. 8 simultaneously the drawdowns in the wells were as follows:

Well No.	ON or OFF	Pump Rate (USgpm)	Water level (ft.)	Drawdown (ft.)
11 5	on off	314	122.30	49.63 2.98
8	on	510	121.38	47.10

At 90 minutes after the start of pumping.Well No. 5 was turned on at a rate of 270 USgpm. With all three wells pumping simultaneously the drawdowns in the wells after 10 minutes of pumping were:

Well No.	ON or	Pump Rate	Water level	Drawdown
	OFF	(USgpm)	(ft.)	(ft.)
11	on	314	123.45	50.78
5	on	270	110.53	38.14
8	on	490	120.12	45.84

When Well No. 8 was turned on it caused an interference effect in Well No. 11 of 1.17 ft. When Well No. 5 was turned on it caused an additional interference effect of 1.15 ft. for a total of 2.32 ft. These interferences are not very significant in terms of the total drawdown, but should be considered in rating the well capacity. The small interference between wells is a result of the relatively high transmissivity of the aquifer indicated by former pump testing of Wells No. 5 and No. 8.

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Well Capacity

The total available drawdown in Well No. 11 is 72.5 ft. or about 70 ft. allowing for the effects of well interference when pumping Wells No. 5 and No. 8. Based on use of 70% of the available drawdown and a specific capacity of 6.48 USgpm/ft. of drawdown, the capacity of Well No. 11 is 317 USgpm. Coincidentally the rate during the final test was 314 USgpm.

DISCUSSION

A comparison of the logs of Wells No. 5, No. 8 and No. 11 shows that it is not possible to correlate between the logs of the wells other than in a general way. The bottom of the aquifer in all three wells is marked by a lithologic change to silt. The water-bearing sand and gravels in the three holes varies in thickness, the thickest section being located in Well No. 8. The bottom of the water-bearing sand and gravel in Wells No. 5, No. 8 and No. 11 is 152 ft., 188 ft. and 166 ft. respectively. All indications are that the performance of Well No. 11 should be considerably better. We believe that the poor performance is probably mostly related to incomplete development. The aquifer is finer at the site of Well 11 however, and the finer screen slot sizes are a reflection of this.

The development carried out to date on the well consists of about one day of bailing by the drilling contractor and one day of backwashing by the pump testing contractor. More work on development is certainly justified. The backwashing technique did not seem to be having any effect near the end so other methods should be tried. From past experience surging has a detrimental effect and should be avoided. A.C. Drillers have proposed using a double packer arrangement in conjunction with a 6" Sochris pump in 6" pipe. This method should be considered in future if development is carried out to improve the performance. The poor performance should not affect the use of the well but the well should obviously be monitored in light of the situation at Well No. 5.

We understand from our discussions with you following the testing of Well No. 11, that at present the size of the main which conveys water to the reservoir is a constriction to the amount of water which can be put into the system at this location. For this reason there is no reason to carry out further development work at present.

SUMMARY

- 1. Well No. 11 is constructed between Wells No. 5 and No. 8 in the pipe yard. It is completed with 22 ft. of screen set from 144 to 166 ft.
- 2. The performance of Well No. 11 is considerably less than that of the two existing wells.
- 3. At present well performance and by utilizing 70% of the available drawdown, the capacity of Well No. 11 is 317 USgpm.
- 4. Poor well performance is probably mostly related to incomplete development, although the aquifer is somewhat finer at this location.
- 5. Further development of Well No. 11 can probably increase its capacity if more water is required for the system in future.

Yours truly, PACIFIC HYDROLOGY CONSULTANTS LTD

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A. Badry, Geologist

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APPENDIX

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MAILINC DDRESS: P.O. BOX 3118 ST. P. _, MINNESOTA = 55165

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SO MANY CONSIDERATIONS ENTER INTO THE MAKING OF A GOOD WELL THAT, WHILE WE BELIEVE SLOT SIZES FURNISHED OR RECOMMENDED FROM SAND SAMPLES ARE CORRECT WE ASSUME NO RESPONSIBILITY FOR THE SUCCESSFUL OPERATION OF JOHNSON WELL SCREENS.

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PUMP TEST WELL NO. 11 IN PIPE YARD

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RUTLAND WATERWORKS DISTRICT

JULY, 1980

		PUMPED No.		OBS. WE No. 5		OBS.WEL			
Time	Mins Since Start	Depth to water (ft.)	Drawdown (ft.)	Depth to water (ft.)	Drawdown (ft.)	Depth to water (ft.)	Drawdown (ft.)	Pumping Rate (USgpm)	Remarks
July 14	ļ								
08:45									Pump off. Wells #5 & #8
July 14	Ļ								
15:50		72.67		72.39		75.25			Static levels
15:50 ¹ 2	1/2	108.03	35.36	70 07	0.00				4" orifice on 6"pipe
15:51	1			73.07	0.68				
15:52 15:53	2 3	124 67	62.00	73.78	1.39				Datum for measure-
15:53	4	134.67 133.19	62.00 60.52	74.23	1.84				ments in all wells
15:54 ¹ / ₂	4 4 ¹ 2	131.48	58.81	74.39	2.00		1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 -		is approximately
15.54^{2} 15:55	τ ² 5	130.05	57.38	74.37	1.98			305	2 ft. above ground Orifice Press = 23"
15:56	6	128.20	55.53	74.36	1.98			305	orrite riess - 23
15:57	7	128.21	55.54	74.33	1.94				
15:58	8	129.04	56.37	74.34	1.95				
15:59	9	129.06	56.39	74.36	1.97				
16:00	10	129.12	56.45	74.39	2.00				
16:02	12	129.16	56.49	74.39	2.00				
16:04	14	129.10	56.43	74.39	2.00	77.00	1.75		
16:06	16	129.22	56.55	74.41	2.02				
16:08	18	129.26	56.59	74.42	2.03				
16:10	20	129.22	56.55	74.42	2.03				
16:15	25	129.42	56.75	74.45	2.06				
16:20 16.25	30 35	129.53	56.86	74.46	2.07	77.07	1.82		
16:25	35 40	129.48 129.48	56.81 56.81	74 40	0.00		•		
16:35	45	129.40	56.85	74.48	2.09				
16:40	40 50	129.52	56.83	74.49	2.10				
16:50	60	129.48	56.81	74.49	2.08				
17:00	70	129.62	56.95	74.48	2.09				
17:10	80	129.51	56.84	74.51	2.12				
17.20	90	129.31	56.82	74.51	2.08	77.09	1.84		
11.40	50	169.49	50.02	/ 4. 4/	2.00	11.09	1.04		

			R	ECOVER	Y 	RUTLAND WA	TERWORKS DIS	TRICT-PAGE	2
		PUMPED WE No. 1		OBS.W		OBS. W No.			
Time	Mins Since Start	Depth to water (ft.)	Drawdown (ft.)	Depth to water (ft.)	Drawdown (ft.)	Depth to water (ft.)	Drawdown (ft.)	Mins since stop	t/t
$17:21 \\ 17:22 \\ 17:22 \\ 17:23 \\ 17:23 \\ 17:23 \\ 17:24 \\ 17:24 \\ 17:25 \\ 17:26 \\ 17:26 \\ 17:27 \\ 17:28 \\ 17:29 \\ 17:30 \\ 17:32 \\ 17:34 \\ 17:36 \\ 17:38 \\ 17:40 \\ 17:40 \\ 17:40 \\ 17:40 \\ 17:40 \\ 17:40 \\ 17:21 \\ 17:22 \\ 17:40 \\ 11:40 \\ 11:4$	91 92 93 93 $_{2^{1_2}}$ 94 94 95 96 97 98 99 100 102 104 106 108 110	77.94 76.38 75.87 75.60 75.39 75.34 75.29 75.25 75.13 75.06 75.02 74.98 74.96 74.91 74.87 74.86 74.85 74.85	5.27 3.71 3.20 2.93 2.72 2.67 2.62 2.58 2.46 2.39 2.35 2.31 2.29 2.24 2.20 2.19 2.18 2.18	74.28	1.89		· · · · ·	$ \begin{array}{c} 1 \\ 2 \\ 2^{1_{2}} \\ 3 \\ 3^{1_{2}} \\ 4 \\ 4^{1_{2}} \\ 5 \\ 6 \\ 7 \\ 8 \\ 9 \\ 10 \\ 12 \\ 14 \\ 16 \\ 18 \\ 20 \\ \end{array} $	91 46 37 31 26.7 23.5 21 19 16 13.86 12.25 11 10 8.5 7.43 6.62 6 5.5
July 16 08:40 08:41 08:42 08:43 08:44 08:45 08:45 08:46 08:47 08:48 08:49	1 2 3 4 5 6 7 8 9	72.67 113.06 117.66 118.97 119.21 119.49 119.64 119.99 120.13 120.07	46.34 46.82 46.97 47.32 47.46 47.40	70.29	PING 2.06	74.28	0.10	pump on in 4" orific	vels; start n Well No. 11 e on 6" pipe ate= 314USgpm
08:50	10	120.29	47.62	72.35	2.06	10.30	0.10		

RUTLAND WATERWORKS DISTRICT PAGE 3

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		PUMPED W No. 1		OBS. WEL No. 5	.L	OBS.WE No.8		
Time	Mins since start	Depth to water (ft.)	Drawdown (ft.)	Depth to water (ft.)	Drawdown (ft.)	Depth to water (ft.)	Drawdown (ft.)	Remarks
08:52	12	120.43	47.76					
08:54	14	120.70	48.03					
08:56	16	120.68	48.01					
08:58	18	120.79	48.12					
09:00	20	120.69	48.02					
09:05	25	120.84	48.17					
09:10	30	120.98	48.31					
09:20	40	121.15	48.48					
09:30	50	121.10	48.43					
09:40	60	121.13	48.46	72.57	2.28	75.47	1.19	Pump #8 on 0 60 mins 0 510 USgpm
09:50	70	122.38	49.71			121.14	46.86	51
10:00	80	122.33	49.66			121.36	47.08	
10:10	90	122.30	49.63	73.27	2.98	121.38	47.10	Pump #5 on 0 90 mins 0 270 USgpm
10:20 10:30	100 110	123.45 123.53	50.78 50.86	110.53	38.14	120.12	45.84	#8 reduced to 490USgpm



102 - 3677 Highway 97N Kelowna, B.C. V1X 5C3

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Telephone (250) 765-9646 Fax (250) 765-3893

CERTIFICATE OF ANALYSIS

July 3, 1997

Rutland Waterworks District Attention: Kevin Reynolds

Sample Identification: Well #11 Date Sampled: June 18, 1997 Date Received: June 18, 1997

	Alkalinity (Total) Aluminum Arsenic Barium Boron Cadmium Calcium Chloride Chromium Color (True) Copper Cyanide Dissolved Solids(Total) Fluoride Hardness(Total) Iron Lead Magnesium Manganese Mercury Molybdenum Nitrate Nitrite pH Potassium Sodium Sulphate Turbidity Uranium Zinc Total Coliform	53 <0.2 <0.01 <0.01 <0.0002 13.9 1.1 <0.01 <5 <0.01 <5 <0.01 <0.010 85 <0.1 51 0.04 0.001 3.98 <0.005 <0.00005 <0.00005 <0.03 0.19 <0.01 6.8 0.71 4.3 6 0.30 0.00011 <0.005	<pre>mg/L as CaCO₃ mg/L mg/L mg/L mg/L mg/L mg/L mg/L mg/L</pre>
Colonies/100mL	Total Coliform Fecal Coliform	<0.005 0 0	mg/L Colonies/100mL Colonies/100mL



RUTLASE SATESTORKS

Certified by:

CARO ENVIRONMENTAL SERVICES Janice M. Fraser, B.Sc., Lab Manager THE INFORMATION CONTAINED IN THIS REPORT IS THE CONFIDENTIAL PROPERTY OF THE CLIENT. ANY LIABILITY ATTACHED THERETO IS LIMITED TO THE FEE CHARGED.



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102 - 3677 Highway 97N Kelowna, B.C. V1X 5C3

Telephone (250) 765-9646 Fax (250) 765-3893

CERTIFICATE OF ANALYSIS

July 3, 1997

Rutland Waterworks District Attention: Kevin Reynolds

Sample Identification: Well #13 Date Sampled: June 18, 1997 Date Received: June 18, 1997

Alkalinity (Total) Aluminum Arsenic Barium Boron Cadmium Calcium Chloride Chromium Color (True) Copper Cyanide Dissolved Solids (Total) Fluoride Hardness (Total) Iron Lead Magnesium Manganese Mercury Molybdenum Nitrate Nitrite Hq Potassium Sodium Sulphate Turbidity Uranium Zinc Total Coliform Fecal Coliform

mg/L as CaCO3 169 <0.2 mg/L <0.01 mg/L <0.01 mg/L <0.1 mg/L <0.0002 mg/L 62.3 mg/L 12.0 mg/L <0.01 mg/L <5 Color Units <0.01 mg/L <0.010 mg/L 307 mg/L 0.2 mg/L mg/L as CaCO3 221 mg/L <0.03 0.001 mg/L 15.8 mg/L <0.005 mg/L <0.00005 mg/L <0.03 mq/L 4.70 mg/L as N mg/L as N <0.01 7.3 pH Units 2.4 mg/L 16.5 mg/L 60 mg/L <0.10 N.T.U. 0.00455 mg/L 0.012 mg/L 0 Colonies/100mL 0

Colonies/100mL

Certified by: CARO FAVIRONMENTAL SERVICES

Janice M. Fraser, B.Sc., Lab Manager

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